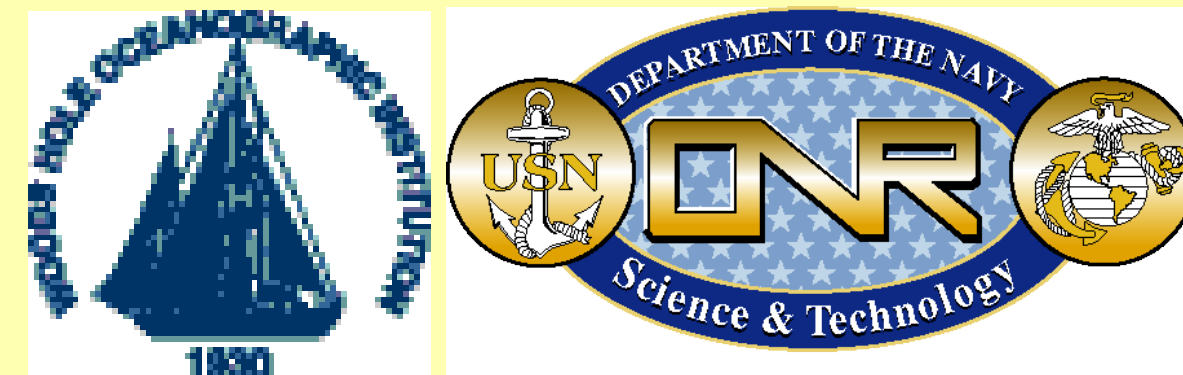


Comparison of Wallingford (DRAMBUIE) Scour Predictions with Measurements



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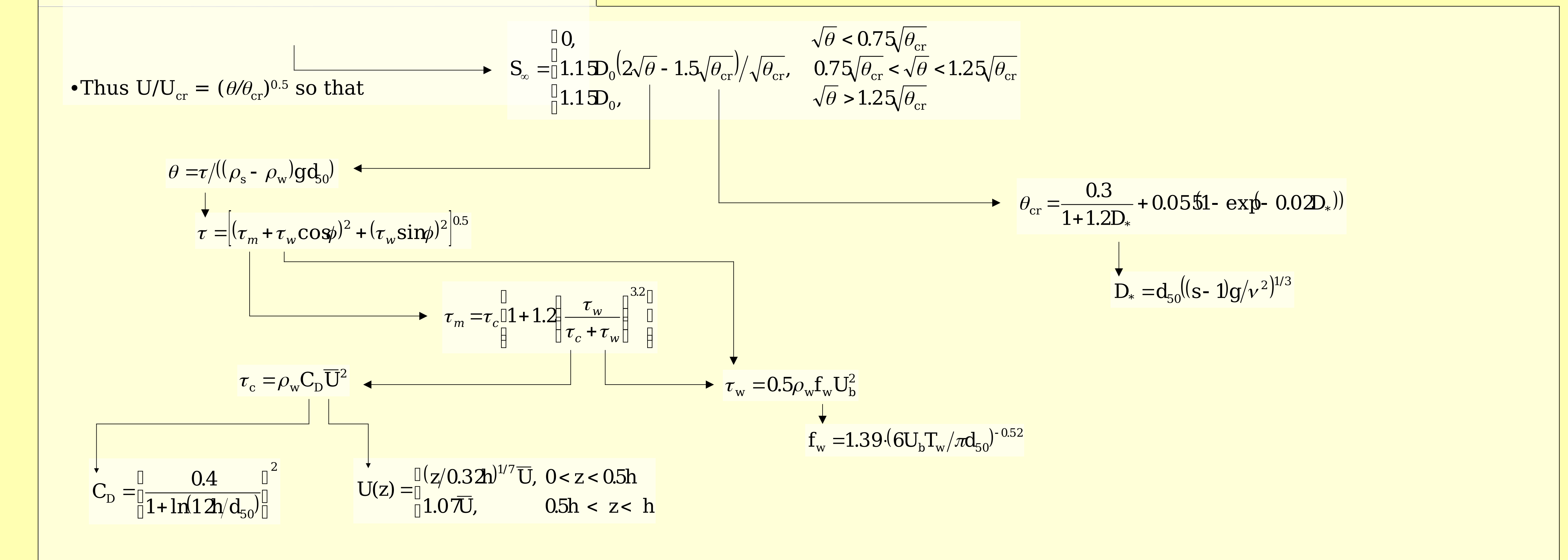
ABSTRACT

The equations that are the basis of the DRAMBUIE scour model were developed at H.R. Wallingford (a U.K. civil engineering firm) and have been published in works by Soulsby (1997) and Whitehouse (1998). We present their equations for predicting scour when both waves and currents are involved and show the predictions with measurements taken from the NRL instrumented mine. The predictions match well with three deployments of the instrumented mine. A fourth deployment is ongoing.

THEORY

A. Main equation: $S(t) = S_c (1 - \exp(-t/T))^p$	List of Symbols A, B = Geometric coeff., A = .095, B = -2.0 B_0 = mine burial depth C_r = drag coefficient for total stress D_0 = mine diameter d_{50} = median grain diameter f_w = wave friction factor g = gravitational acceleration h = water depth p = 0.6, geometric parameter S = scour pit depth at time t S_c = scour pit depth at $t = 0$ $s = \rho_s / \rho_w$ T = time constant for rate of scour pit growth t = time U = mean velocity at the seabed $\bar{u} =$ depth averaged current velocity
<ul style="list-style-type: none">$S(t)$ = depth of scour pit at time t S_c = scour pit depth at $t = 0$ T = time constant governing rate of growth p = 0.6, geometric parameterS_c is found by the equations in Block BT is found by the equations in Block CTime stepping and use for burial depth is shown in Block DBurial depth is the maximum scour depth over the deployment period minus the amount of re-exposure $B_0(t) = \max(S(t)) - \alpha S(t)$	<ul style="list-style-type: none">U_b = mean orbital velocity U_{cr} = critical velocity for sediment transport T_w = mean wave period z = sensor depth $\alpha = 0.6$, efficiency for scour to re-expose a buried mine θ = Shield's parameter θ_{cr} = critical Shield's parameter for sediment transport θ_0 = Shield's parameter when no mine is present ρ_s = sand density ρ_w = water density τ = stress τ_c = stress due to current alone τ_{cr} = critical stress for sediment transport τ_m = nonlinearly combined stress from τ_c and τ_w τ_w = stress due to waves ϕ = angle between current and waves

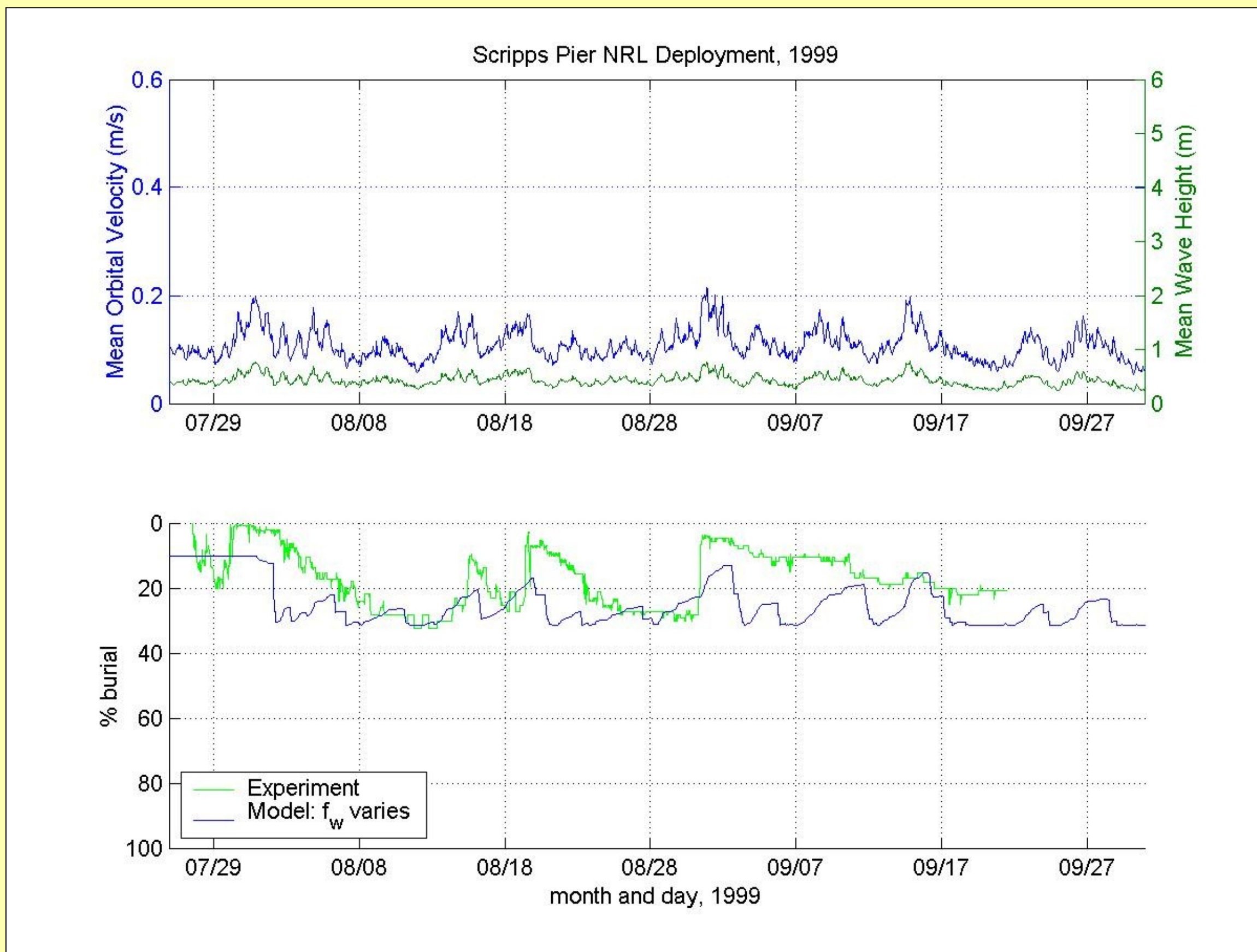
B. Obtaining S_c: <ul style="list-style-type: none">S_c is found from<ul style="list-style-type: none">$0 \leq U < 0.75U_{cr}$ $S_c = 1.15D_0(2U - 1.5U_{cr})/U_{cr}$$0.75U_{cr} \leq U < 1.25U_{cr}$ $S_c = 1.15D_0$$U \geq 1.25U_{cr}$ $S_c = 1.15D_0$U can be measured directly, but U_{cr} is not directly obtainableChange variables in S_r from U and U_{cr} to Shield's parameter at the bed is θ and θ_{cr} which can be evaluated $\tau = \rho_w C_r U^2$The Shield's parameter is $\theta = \tau / ((\rho_s - \rho_w)gd_{50})$At U_{cr} $\tau \rightarrow \tau_{cr} = \rho_w C_r U_{cr}^2$ $\theta \rightarrow \theta_{cr} = \tau_{cr} / ((\rho_s - \rho_w)gd_{50})$	C. Obtaining T: <ul style="list-style-type: none">Time constant has been empirically determined to be $T = T^* ((s - 1)gd_{50}^{-1/2} D_0^2)$ where $T^* = A\theta^B$$A = 0.095$ and $B = -2.02$ for a 5:1 cylinder, thus $T = A\theta^B ((s - 1)gd_{50}^{-1/2} D_0^2)$	D. Time Stepping and Burial Depth: <ul style="list-style-type: none">Rewrite the Main Equation in Block A as a time stepped calculation. For the $j+1^{th}$ time interval, $S_{t_{j+1}} = S_c(t_j)(1 - \exp(-(t + \Delta t)/T(t_j))^p)$ where the steady-state time to get to $S(t_j)$ is $t = T(t_j)(-\ln(1 - S(t_j)/S_c(t_j)))^{1/p}$Set an upper limit to the scour depth for the $j+1^{th}$ step $S_{t_{j+1}} = \begin{cases} S_c(t_j)(1 - \exp(-(t + \Delta t)/T(t_j))^p), & S(t_j) < S_c(t_j) \\ S_c(t_j), & S(t_j) \geq S_c(t_j) \end{cases}$Burial is the maximum scour depth achieved minus the re-exposure. $B_0(t_j) = \max(S(t \leq t_j)) - \alpha S(t_j)$
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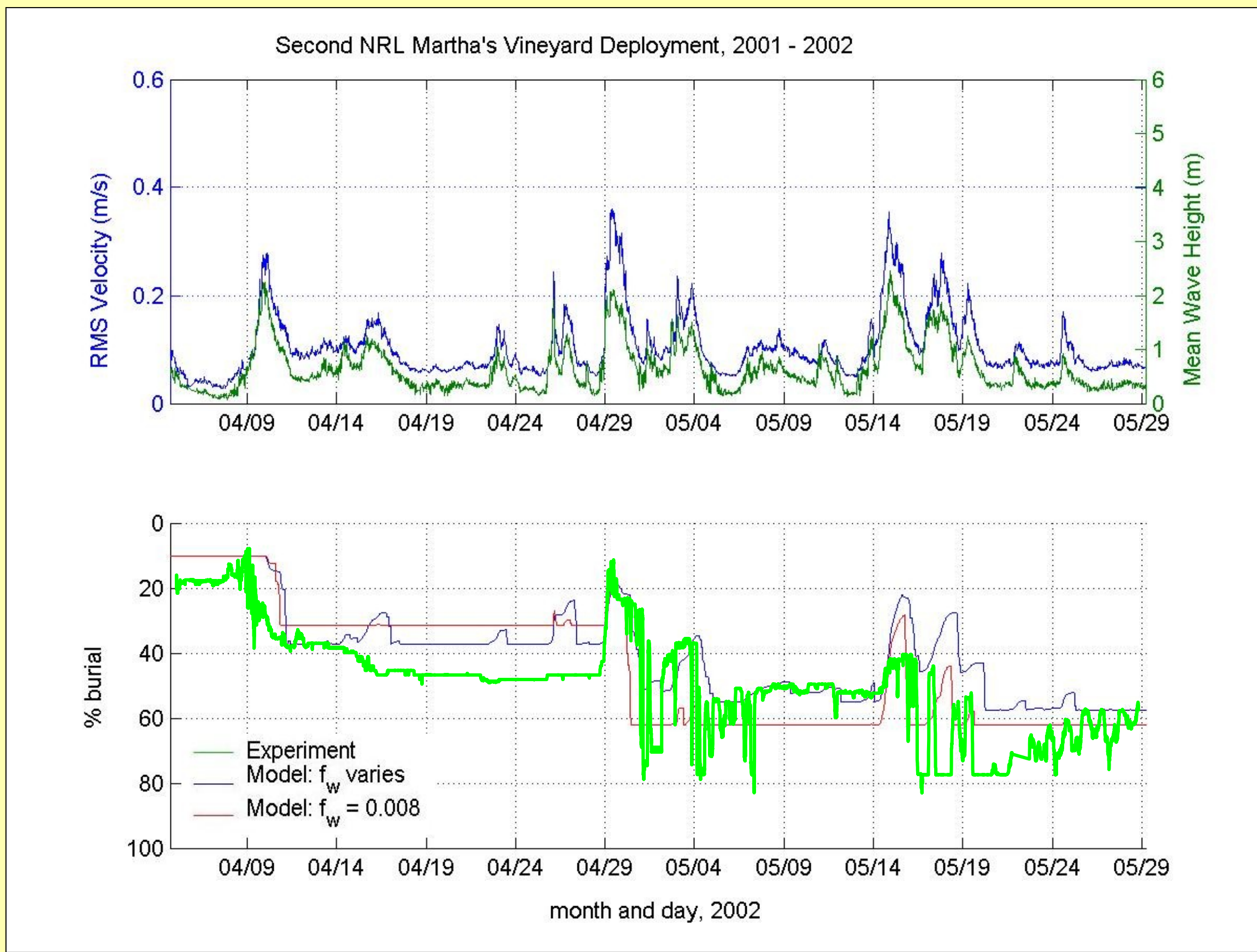
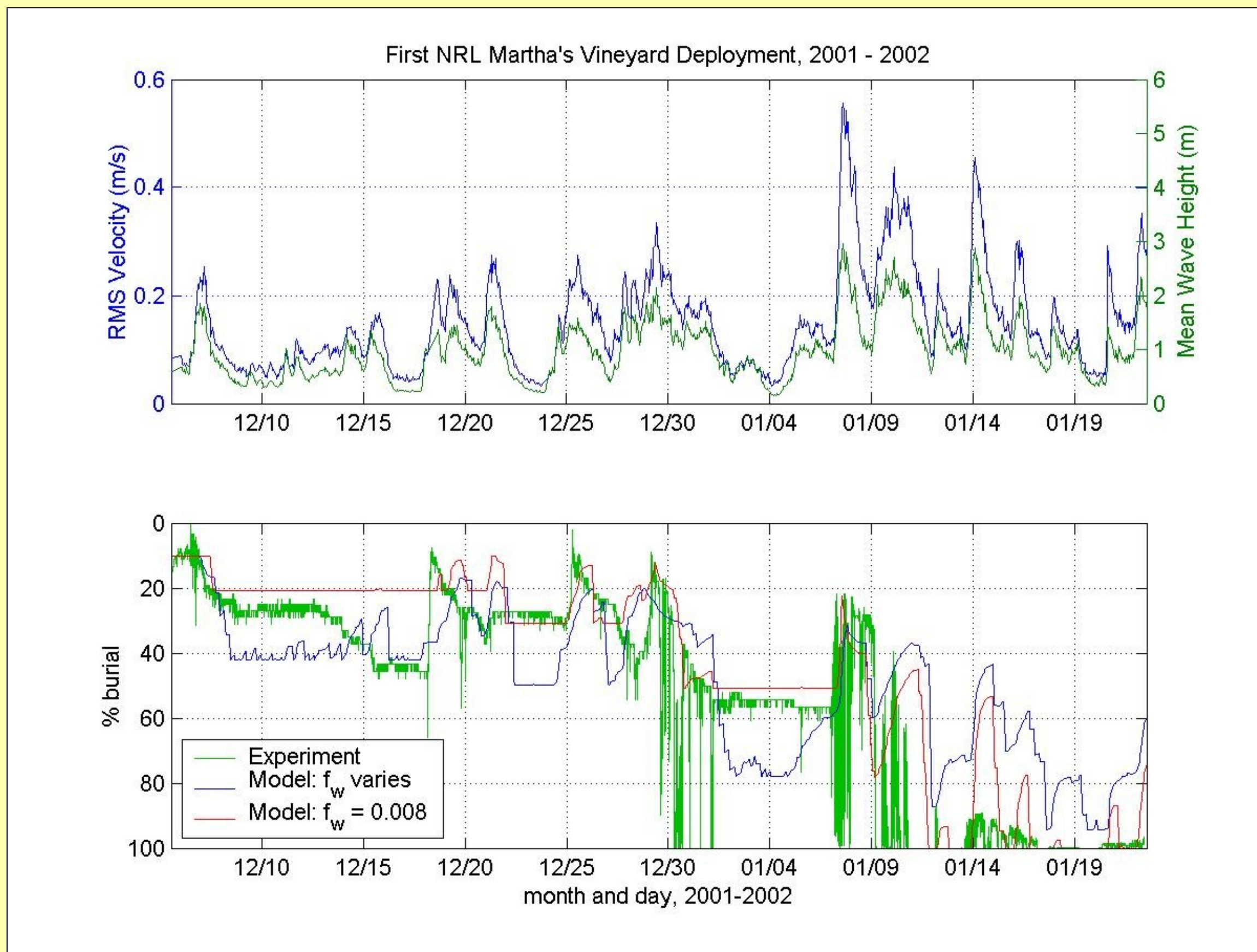
COMPARISON OF PREDICTION WITH MEASUREMENT



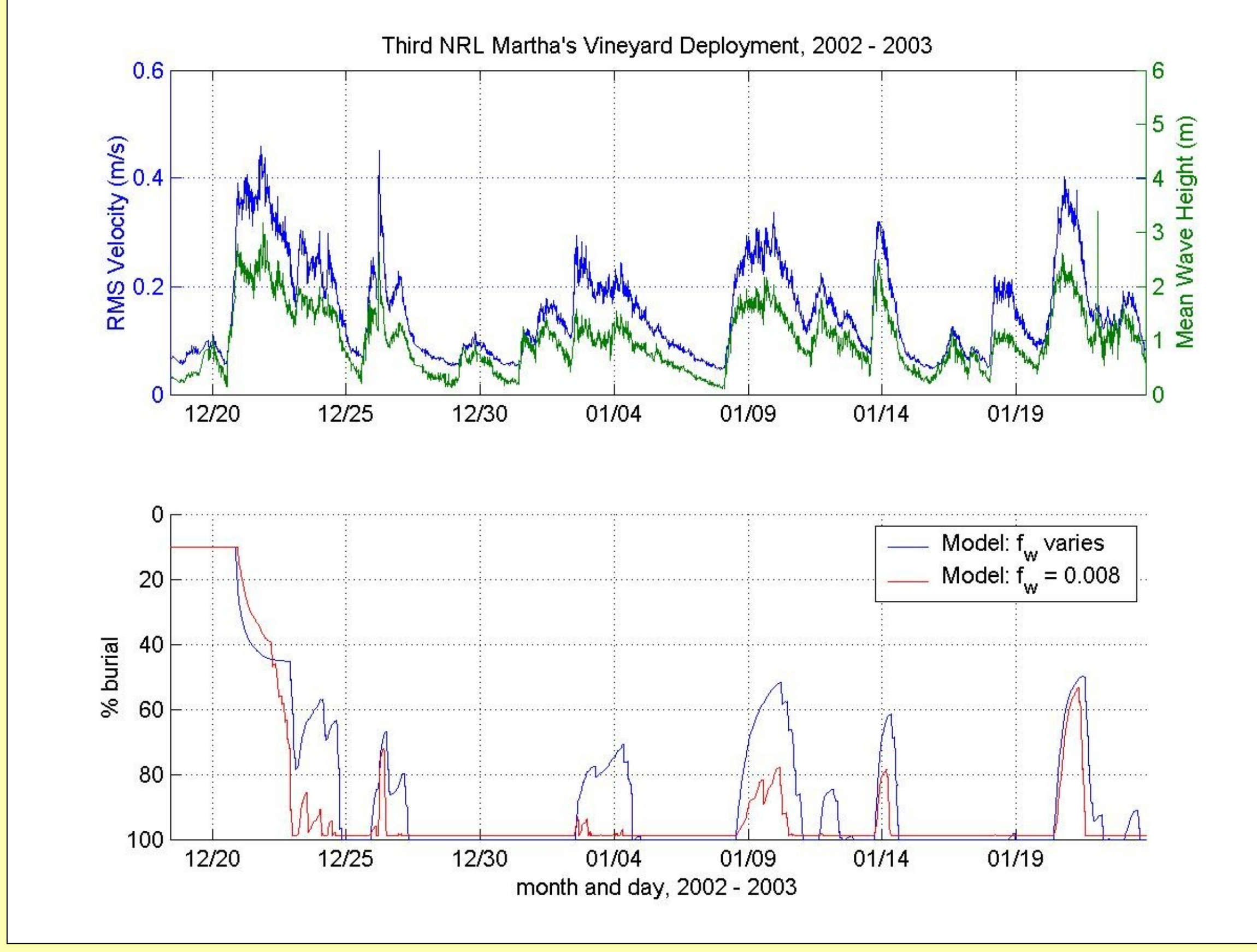
The NRL instrumented mine being deployed at Martha's Vineyard Coastal Observatory (MVCO).



Completed deployment at the Scripps Institute of Oceanography, Summer 1999. Median grain diameter = 0.19 mm. First completed deployment at Martha's Vineyard Coastal Observatory, Winter 2001-2002. Median grain diameter = 0.18 mm. Model run for varying f_w only.



Second completed deployment at Martha's Vineyard Coastal Observatory, Spring 2002. Median grain diameter = 0.18 mm. Third deployment (predictions only) at Martha's Vineyard Coastal Observatory, Winter 2002-2003. Median grain diameter = 0.60 mm. This deployment is still in progress.



CONCLUSIONS

The HR Wallingford equations appear to be promising for predicting the scour of a free body cylinder in sand with grain sizes on the order of 0.2 mm. We still need to compare the model output with the mine data for the third MVCO deployment, which is on coarser sand (0.5 mm), and evaluate the usefulness of including tidal currents. The model is simple and mature enough to be considered for integration into first generation holistic prediction systems.

Further work is required, however, to make the model more applicable to a wider variety of mines. At the moment, the geometrically determined parameters in the equations are given only for 5:1 cylinders. Work is required to examine the sensitivity of the model output to uncertainty of these parameters, and determine what these parameters may be for other mine geometries if a significant sensitivity is detectable. Another refinement that is needed is to account for the decreasing cross-section of the mine above the seabed as it scours into the sediment instead of the currently working assumption of a constant cross-section.

REFERENCES

Soulsby, R. (1997) *Dynamics of Marine Sands: A Manual for Practical Applications*, Thomas Telford: London.
Whitehouse, R. (1998) *Scour at Marine Structures: A Manual for Practical Applications*, Thomas Telford: London.

ACKNOWLEDGEMENTS

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